April 16, 2026 10:42:09 AM

## LIST OF DRAWINGS


S1.01 GENERAL NOTES

S2.01 VGH BIKE STORAGE SHELTER EXPANSION

S3.01 SECTIONS


## GENERAL NOTES

1. THE DESIGN INDICATED ON THESE DRAWINGS HAS BEEN DESIGNED IN GENERAL ACCORDANCE WITH THE BRITISH COLUMBIA BUILDING CODE 2024 (B.C.B.C. 2024) WITH THE FOLLOWING PARAMETERS.

2. AUTHORITY HAVING JURISDICTION: TOWN OF VIEW ROYAL
   IMPORTANCE CATEGORY: NORMAL
   Ss = 1.5 Sr = 0.3
   WIND LOAD: q50 = 11.9 kPa

   SEISMIC PARAMETERS:
   Rd = 2.0 SITE CLASS = D
   Ro = 1.3 $Sa(0.2,X_D)$ = 1.89
   $Sa(0.5,X_D)$ = 2.0
   $Sa(1.0,X_D)$ = 1.44
   $Sa(2.0,X_D)$ = 0.933
   $Sa(5.0,X_D)$ = 0.248
   $Sa(10.0,X_D)$ = 0.0797

3. DESIGN LOADS:

| | LIVE LOAD (kPa) | SUPER IMPOSED DEAD LOAD (kPa) | CONCENTRATED POINT LOAD (kN) |
|---|---|---|---|
| ROOFS | 0.55 Ss + Sr + BUILT-UP SNOW | 1.0 (TOTAL D.L.) | - |

4. REFER TO ARCHITECTURAL, MECHANICAL & ELECTRICAL DRAWINGS FOR PENETRATIONS THRU STRUCTURAL MEMBERS.

5. REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONS, ELEVATIONS, SLOPES, CURBS, FIRE RATINGS, ETC.

6. SKYLINE ENGINEERING IS NOT RESPONSIBLE FOR THE DESIGN, OR FIELD REVIEW OF NON STRUCTURAL ELEMENTS SUCH AS WINDOWS, HANDRAILS, GUARDRAILS, PRECAST CLADDING, BRICK TIES, CLADDING, ROOFING, ETC. THESE ELEMENTS ARE TO BE DESIGNED AND DETAILED BY A SPECIALTY STRUCTURAL ENGINEER LICENSED IN B.C. DETAILED WIND OR SNOW LOAD CALCULATIONS FOR THE DESIGN OF THESE ELEMENTS IS TO BE PERFORMED BY THE SPECIALTY ENGINEER.

7. NON STRUCTURAL ELEMENTS THAT EFFECT THE BASE STRUCTURE SHALL BE REVIEWED BY SKYLINE ENGINEERING LTD. PLEASE SUBMIT SHOP DRAWINGS FOR OUR REVIEW OF SUCH ELEMENTS.

8. SKYLINE ENGINEERING IS NOT RESPONSIBLE FOR SHORING OF THE STRUCTURE DURING CONSTRUCTION OR OF THE SOILS DURING EXCAVATION.

## FIELD REVIEW BY SKYLINE ENGINEERING

1. SKYLINE ENGINEERING PROVIDES FIELD REVIEW ONLY FOR THE WORK SHOWN ON THESE STRUCTURAL DRAWINGS. THIS REVIEW IS CONDUCTED WITH SUCH FREQUENCY AS SKYLINE ENGINEERING DEEMS APPROPRIATE TO OBSERVE VARIOUS STAGES OF THE WORK AND TO ASCERTAIN THAT THE WORK IS IN GENERAL CONFORMANCE WITH THE PLANS AND SUPPORTING DOCUMENTS PREPARED BY SKYLINE ENGINEERING. FIELD REVIEW BY SKYLINE ENGINEERING IS NOT CARRIED OUT FOR THE CONTRACTOR'S BENEFIT, NOR DOES IT MAKE SKYLINE ENGINEERING GUARANTORS OF THE CONTRACTOR'S WORK, AND MAY NOT FORM PART OF THE CONTRACTORS CONSTRUCTION QUALITY CONTROL WHICH SHALL REMAIN THE RESPONSIBILITY OF THE CONTRACTOR(S). IT REMAINS THE CONTRACTOR'S RESPONSIBILITY TO BUILD THE WORK IN CONFORMANCE WITH THE CONTRACT DOCUMENTS. SKYLINE ENGINEERING SHALL NOT BE RESPONSIBLE FOR THE ACTS OR OMISSIONS OF THE CONTRACTOR, SUB-CONTRACTOR, OR ANY OTHER PERSONS PERFORMING ANY OF THE WORK OR FOR THE FAILURE OF ANY OF THEM TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

2. THE CONTRACTOR(S) SHALL GIVE NOTICE THAT APPROPRIATE PORTIONS OF THE WORK ARE COMPLETE AND AVAILABLE FOR FIELD REVIEW. IT IS THE CONTRACTORS RESPONSIBILITY TO COORDINATE FIELD REVIEWS IN A TIMELY MANNER SUITABLE TO THE METHODS AND SCHEDULE OF CONSTRUCTION. WORK COVERED BY FINISHES PRIOR TO FIELD REVIEW BY THE CONSULTANT OR BY SPECIALTY ENGINEERS INCLUDING CONCRETE CAST AROUND REBAR MAY REQUIRE REMOVAL IN ORDER TO REVIEW THE WORK. THE COST OF REMOVAL FOR REVIEW PURPOSES SHALL BE BOURN BY THE CONTRACTOR. INSTRUCTIONS FOR REMOVAL OF FINISHES OR CONCRETE ARE AT THE SOLE DISCRETION OF THE STRUCTURAL ENGINEER OF RECORD.

3. THE CONTRACTOR SHALL PROVIDE AT LEAST 24 HOURS. ADVANCE NOTICE ON A BUSINESS DAY OF EACH REQUIRED FIELD REVIEW. FIELD REVIEWS SHALL BE DURING NORMAL WORKING HOURS ONLY. FIELD REVIEWS AT OTHER TIMES REQUIRE 72 HR. NOTICE AND SHALL BE PAID BY THE CONTRACTOR. FIELD REVIEWS REQUIRING SUBSTANTIAL TRAVEL TIME MUST BE GIVEN ADEQUATE NOTICE.

4. INSTRUCTIONS GIVEN AS A RESULT OF FIELD REVIEW SHALL NOT BE CAUSE FOR EXTRA CHARGE TO THE CONTRACT.

5. WORK MUST BE GENERALLY COMPLETE AT THE TIME OF REVIEW.

## WOOD FRAMING

GENERAL:

1. WOOD FRAMING TO BE CONSTRUCTED IN ACCORDANCE WITH THE CURRENT EDITION OF THE BRITISH COLUMBIA BUILDING CODE (BCBC) AND CSA -086. MINIMUM STANDARD FOR FRAMING IN ACCORDANCE WITH PART 9 OF BCBC.

2. NO NOTCHING OR DRILLING OF JOISTS AND BEAMS ALLOWED WITHOUT EXPRESS PERMISSION OF THE ENGINEER OF RECORD. HOLES THROUGH JOISTS AND BEAMS TO FOLLOW RECOMMENDATION IN PART 9 OF BCBC.

3. NOTCHING OR DRILLING OF LOAD BEARING STUDS TO BE AS PER THE FOLLOWING:
   a. TO FOLLOW RECOMMENDATIONS IN PART 9 OF BCBC.
   b. MAXIMUM DEPTH OF NOTCH NOT TO EXCEED 1/4 OF STUD DEPTH.
   c. MAXIMUM HEIGHT OF NOTCH NOT TO EXCEED 89mm.
   d. HOLES TO BE CENTERED IN THE STUD DEPTH, WITH MAXIMUM HOLE DIAMETER NOT TO EXCEED 1/3 OF STUD DEPTH.

4. ALL TIMBER FRAMING SHALL HAVE GRADE STAMPS AND NON-GRADED WOOD WILL BE REJECTED.

5. ALL WOOD IN CONTACT WITH CONCRETE SHALL BE PRESSURE TREATED OR PROVIDE SILL GASKET MATERIAL BETWEEN WOOD AND CONCRETE.

6. PROVIDE 2-PLY 38x235 DROP BEAMS OVER ALL OPENINGS U.N.O. ON PLAN.

7. WOOD FRAME SHRINKAGE:
   a. WOOD FRAME STRUCTURES WILL ENDERGO MOVEMENT DUE TO SHRINKAGE, SETTLEMENT, WARPING, AND TWISTING WHICH IS INHERENT TO WOOD FRAME STRUCTURES.
   b. SHRINKAGE OF WOOD PRODUCTS OVER TIMESHALL BE ACCOUNTED FOR IN ALL ELECTRICAL, MECHANICAL, AND INTERIOR/EXTERIOR FINISHES.
   c. ALL TIMBER SHALL BE KILN DRIED AND HAVE A MAXIMUM MOISTURE CONTENT OF 19% WHEN IT ARRIVES ON SITE.
   d. ALL WOOD MATERIALS SHALL BE REASONABLY PROTECTED FROM THE ENVIRONMENT AND/OR MOISTURE WHILE STORED ON SITE, AND DURING THE CONSTRUCTION PROCESS.
   e. WOOD FRAME SHRINKAGE MINIMUM ALLOWANCES :
      i. ENGINEERED FLOOR SYSTEMS: 13mm PER FLOOR
      ii. DIMENSIONAL FLOOR SYSTEMS: 25mm PER FLOOR
   f. BUILDINGS CONSTRUCTED OF 5 OR 6 STORIES OF CONTINUOUS WOOD FRAME CONSTRUCTION ARE TO HAVE A MOISTURE CONTENT OF 12% OR LESS AT THE TIME FINISHES (SUCH AS GYPSUM WALL BOARD) ARE INSTALLED. REFER TO ARCHITECTURAL, AND BUILDING ENVELOPE DRAWINGS AND SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.

POSTS:

1. UNLESS NOTED OTHERWISE ON PLAN PROVIDE BUILT-UP 2 x POST AT THE ENDS OF ALL BEAMS AS FOLLOWS:

   BEAMS PARALLEL TO WALL:
   a. DROP BEAM WITH SPAN LESS THAN 1200 = 1 CRIPPLE + 1 FULL HEIGHT (2-TOTAL)
   b. DROP BEAM WITH SPAN GREATER THAN 1200 = 2 CRIPPLE + 1 FULL HEIGHT (3-TOTAL)
   c. FLUSH BEAM WITH SPAN LESS THAN 1200 = 2-STUDS
   d. FLUSH BEAM WITH SPAN GREATER THAN 1200 = 3-STUDS

   BEAMS PERPENDICULAR TO WALL:
   a. DROP BEAMS: TO MATCH WIDTH OF BEAM PLUS ONE FULL HEIGHT STUD EACH SIDE
   b. FLUSH BEAMS: TO MATCH WIDTH OF BEAM

2. BUILT UP 38x89 AND 38x140 POSTS SHALL BE NAILED TOGETHER WITH 2-ROWS OF 76mm NAILS @ 250 O.C. TYPICAL.

3. PROVIDE SOLID THRU FLOOR BLOCKING TO MATCH SIZE OF POST ABOVE.

WALLS:

1. WALL STUD SPACING SHALL NOT EXCEED 400 O.C. U.N.O.

2. NOTCHING OR DRILLING OF LOAD BEARING STUDS TO BE AS PER NOTE GENERAL: 3.

3. NON-LOAD BEARING WALLS TO BE FRAMED WITH A MINIMUM OF 38x89 @ 400 O.C.

4. ALL LOAD BEARING WALLS NOT SHEATHED WITH PLYWOOD SHEATHING SHALL BE BLOCKED AT MID-HEIGHT OR AT MAX 1372 O.C. EACH STUD SPACE.

MATERIALS:

1. SAWN LUMBER SHALL SURFACED FOUR SIDES TYPICAL AND MEET THE FOLLOWING MINIMUM GRADES U.N.O (SEE ALSO WOOD SHEARWALL REQUIREMENTS).
   a. STUDS SHALL BE S.P.F NO.2 GRADE MINIMUM OR HEM-FIR NO.2 GRADE MINIMUM.
   b. POSTS SHALL BE S.P.F NO.2 GRADE MINIMUM
   c. JOISTS SHALL BE S.P.F NO.2 GRADE MINIMUM
   d. SILL PLATES SHALL BE S.P.F NO.2 GRADE MINIMUM

2. ALL ANCHOR BOLTS AND CONNECTION BOLTS SHALL BE GRADE A36 OR A304 AS A MINIMUM.

3. SILL BOLTS TO BE 12.7mmØ CAST IN J-BOLTS @ 1200 O.C. WITH 125 EMBEDMENT, TYPICAL UNLESS NOTED OTHERWISE. SEE SHEAR WALL SCHEDULE FOR SILL BOLTS IN SHEAR WALLS. KWIK BOLTS OR POST INSTALLED EPOXY ANCHORS MAY BE SUBSTITUTED FOR J-BOLTS IF 100 OR GREATER EDGE DISTANCE TO CONCRETE EDGE. CONFIRM WITH ENGINEER OF RECORD.

4. ALL NAILS SPECIFIED ON DRAWINGS AND SCHEDULES SHALL BE COMMON SIZE NAILS CONFORMING TO THE TABLE LISTED BELOW:

| LENGTH | MINIMUM DIAMETER |
|---|---|
| 2" (51mm) | 0.113" (2.87mm) |
| 2½" (64mm) | 0.131" (3.33mm) |
| 3" (76mm) | 0.144" (3.66mm) |
| 3½" (89mm) | 0.160" (4.06mm) |

5. TIMBER CONNECTION HARDWARE SHALL BE SIMPSON STRONG TIE U.N.O. ALL EXTERIOR HARDWARE SHALL BE HOT DIPPED GALVANIZED OR SIMPSON STRONG TIE - ZMAX COATING. REFER TO MANUFACTURE RECOMMENDATION FOR FASTENER INSTILLATION REQUIREMENTS.

6. SHEATHING SHALL BE D-FIR GRADE SHEATHING AND PLACED IN STAGGERED PATTERN TYPICAL FOR FLOORS AND WALLS. MINIMUM NAILING FOR SHEATHING SHALL BE 64mm NAILS AT 150 O.C. AT PANEL EDGES AND 300 O.C. AT INTERIOR PANEL SUPPORT FRAMING. SHEATHING SHALL CONFORM TO THE FOLLOWING MINIMUM STANDARDS U.N.O. :

   a. ROOF SHEATHING
      i. SLOPE >= TO 15 DEGREES SHALL BE 12.7mm PLYWOOD WITH H-CLIPS AT UNSUPPORTED EDGES.
      ii. SLOPE < TO 15 DEGREES SHALL BE 16mm T&G PLYWOOD

   b. FLOOR SHEATHING
      i. WITH CONCRETE TOPPING AND 16mm PLYWOOD
      ii. WITHOUT CONCRETE TOPPING AND 16mm T&G PLYWOOD

   c. EXTERIOR WALLS.
      i. EXTERIOR CLADDING ATTACHED TO STUDS - 9.5mm PLYWOOD
      ii. EXTERIOR MASONRY, Z-BARS, OR STUCCO ATTACHEMENT - 12.7mm PLYWOOD MINIMUM
      iii. REFER TO ARCHITECTURAL DRAWINGS AND CLADDING SHOP DRAWINGS FOR ADDITIONAL MINIMUM EXTERIOR WALL SHEATHING REQUIREMENTS.

ENGINEERED WOOD PRODUCTS:

1. JOIST SIZING OR DIRECTION SHALL NOT VARY FROM THE STRUCTURAL PLANS WITHOUT PRIOR APPROVAL.

2. ALL HANGERS FOR BEAMS AND JOISTS NOT SPECIFIED ON PLAN SHALL BE CAPABLE OF ACHIEVING 100% OF MEMBER SHEAR CAPACITY.

## CONCRETE ANCHORS

1. PROVIDE ANCHORS AT ALL DRILLED ANCHORS AND REINFORCING STEEL LOCATIONS. INDICATE ON STRUCTURAL DRAWINGS TO DEPTHS OF EMBEDMENT INDICATED.

2. ADHESIVE TO BE HILTI HIT RE 500V3 INJECTION ADHESIVE UNLESS OTHERWISE NOTED ON DRAWINGS.

3. INSTALLATION OF ADHESIVE SHALL BE PERFORMED IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS.

4. DRILLING SHALL BE PERFORMED WITH A ROTARY HAMMER DRILL U.N.O.

5. TRAIN ALL PERSONNEL INVOLVED IN INSTALLATION OF ANCHORS BY HILTI. INSTALLERS TO PROVIDE PROOF OF TRAINING UPON REQUEST.

6. INTENTIONALLY ROUGHEN THE INTERFACE BETWEEN POURS TO 6.4mm AMPLITUDE.

7. ALTERNATIVE FASTENING SYSTEMS PROPOSED BY THE CONTRACTOR SHALL BE SUBMITTED TO THE CONSULTANT FOR REVIEW AND APPROVAL.

## STRUCTURAL STEEL

1. DESIGN OF STRUCTURAL STEEL MEMBERS AND CONNECTIONS TO BE IN ACCORDANCE WITH CAN/CSA S16-01 R09.

2. SHOP DRAWINGS OF STRUCTURAL STEELWORK TO BE SUBMITTED FOR REVIEW PRIOR TO FABRICATION. PROVIDE LETTERS OF ASSURANCE SB/SC (INCLUDING FIELD REVIEW) AND SHOP DRAWINGS SHALL BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE PROVINCE OF BRITISH COLUMBIA FOR CONNECTIONS AND WELDING DESIGNS.

3. SHOP DRAWINGS MUST BE PREPARED USING 3D SHOP DRAWING MODEL HAND DRAWN SHOP DRAWING WILL NOT BE ACCEPTED.

4. WORKMANSHIP AND FABRICATION TO BE IN ACCORDANCE WITH CAN/CSA S16.

5. WELDING SHALL ONLY BE UNDERTAKEN BY A COMPANY CERTIFIED BY THE CANADIAN WELDING BUREAU TO THE REQUIREMENTS OF CSA STANDARD W47J (LATEST EDITION) "CERTIFICATION OF COMPANIES FOR FUSION WELDING OF STEEL" IN DIVISION 1 OR 2 ALL WELDING SHALL BE DONE IN ACCORDANCE WITH AND MEET THE QUALITY REQUIREMENTS OF CSA STANDARD W9 (LATEST EDITION)

6. STRUCTURAL WIDE-FLANGE MEMBERS TO CONFORM TO CSA G4021-M, GRADE 350W

7. STRUCTURAL STEEL ANGLES, PLATES, WELDED WIDE FLANGES, AND CHANNELS TO CONFORM TO CSA G4021-M GRADE 300W

8. HOLLOW STRUCTURAL SECTIONS SHALL CONFORM TO CSA G4021-M, GRADE 350W CLASS C FOR SQUARE AND RECTANGULAR SECTIONS AND A-500, CLASS 'C' (Fy=307 MPA) FOR ROUND SECTIONS. DRY AND SEAL HOLLOW STRUCTURAL SECTIONS IN THE FABRICATION SHOP PRIOR TO SHIPPING TO THE SITE.

9. UNLESS NOTED OTHERWISE ALL BOLTS TO BE HIGH TENSILE BOLTS AND SHALL CONFORM TO ASTM A325 AND ASTM A490. USE CORROSION INHIBITING PLATED BOLTS ON ALL PRE-PAINTED STEEL AND ARCHITECTURAL EXPOSED STEEL.

10. ALL EXTERIOR EXPOSED STEEL, BOLTS, NUTS, AND WASHERS TO BE HOT DIPPED GALVANIZED UNLESS NOTED OTHERWISE.

11. ALL EXPOSED STRUCTURAL STEEL IS TO BE OF ARCHITECTURAL QUALITY. CONNECTION DETAILS, LAYOUTS, WELDING, BOLTING PATTERNS, AND FIT-UP TO BE REVIEWED BY THE ARCHITECT UPON COMPLETION OF SHOP DRAWINGS AND APPROVED UPON COMPLETION OF ERECTION.

12. FOR PAINTING OF EXPOSED STEEL, SEE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS. PRIMER TO BE COMPATIBLE WITH SPECIFIED PAINT FINISH.

13. TEMPORARILY BRACE AND SHORE STEELWORK UNTIL ALL BRACING, BRIDGING SUPPORTS AND DECKING HAVE BEEN INSTALLED. IT IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO ENSURE THAT TEMPORARY BRACING IS ADEQUATE TO RESIST LATERAL LOADS AND PROVIDE STABILITY TO STRUCTURAL MEMBERS DURING CONSTRUCTION.

14. ALL STANDARD AND TYPICAL DETAILS SHOWN ON THE DRAWINGS APPLY TO ALL STEELWORK WHETHER SPECIFICALLY REFERENCED ON PLANS OR NOT.

15. ALL BEAM CONNECTIONS TO BE DESIGNED FOR 50% OF THE BEAM SHEAR CAPACITY TYPICAL UNLESS NOTED OTHERWISE.


SKYLINE ENGINEERING

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| NO | DATE | REVISION |
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| 1 | 2026 04 16 | Issued for Construction |

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SKYLINE ENGINEERING LTD.
PERMIT TO PRACTICE
NO. 1001306

PROFESSIONAL ENGINEER — PROVINCE OF BRITISH COLUMBIA — G. W. GRIFFIN — SCIENCE & ENGINEERING

2026-04-16

PROJECT NAME:
VGH
BIKE STORAGE
SHELTER EXPANSION

SHEET TITLE:
GENERAL NOTES

PROJECT NO:
11630.12

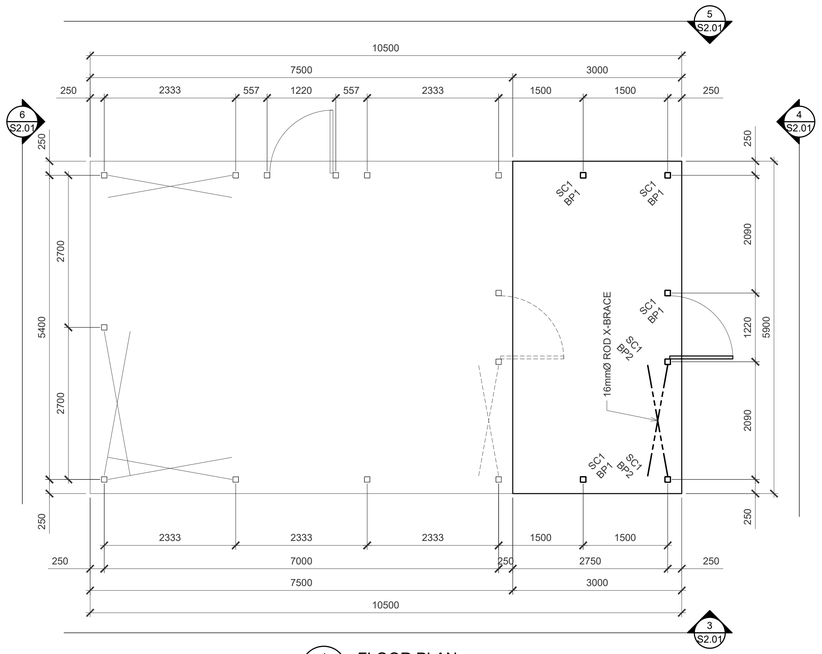
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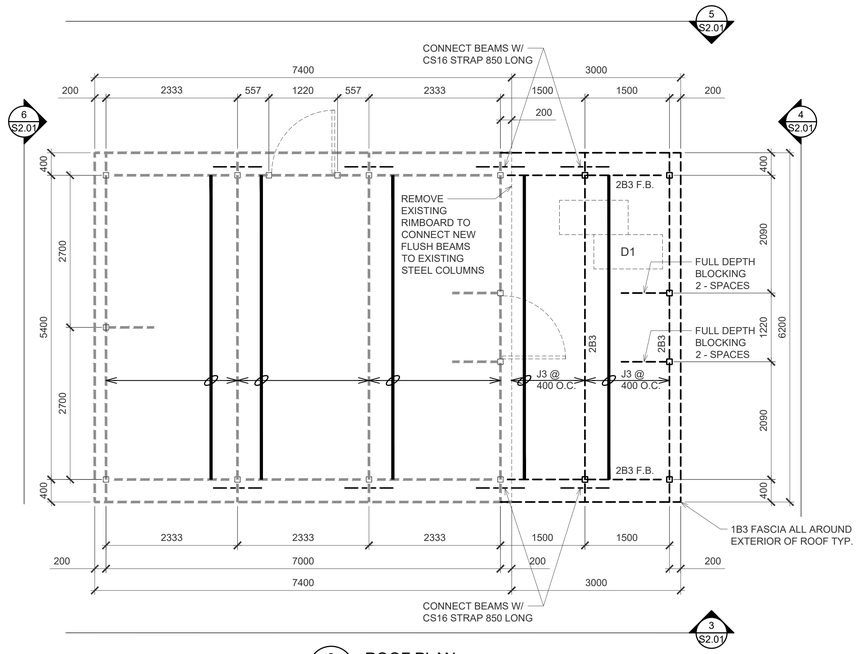
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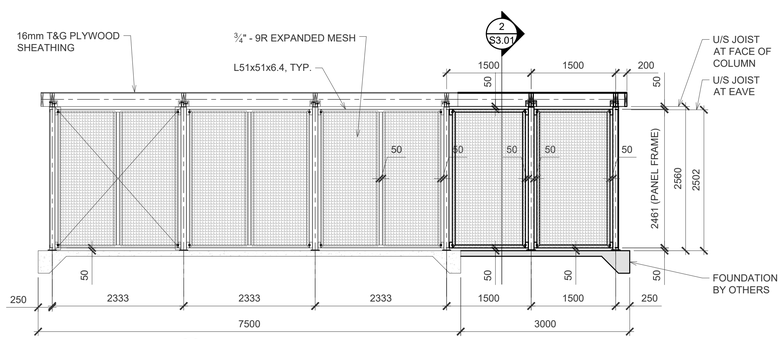
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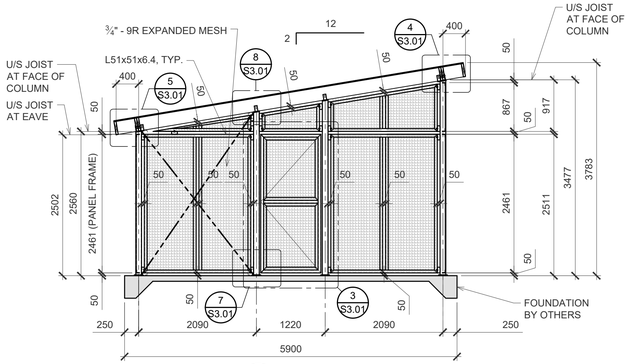

1 S2.01 FLOOR PLAN
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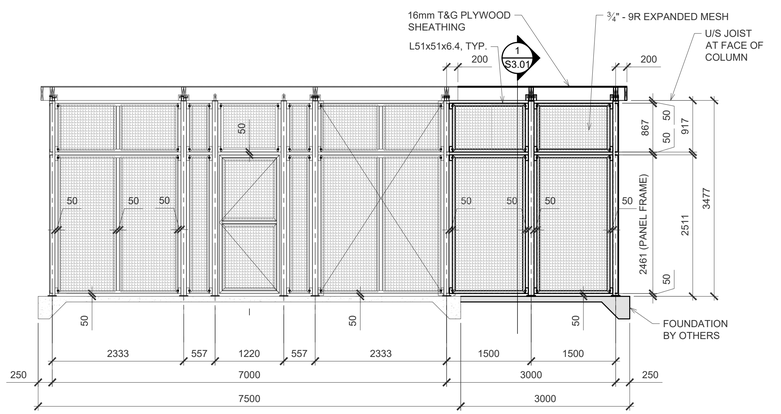

2 S2.01 ROOF PLAN
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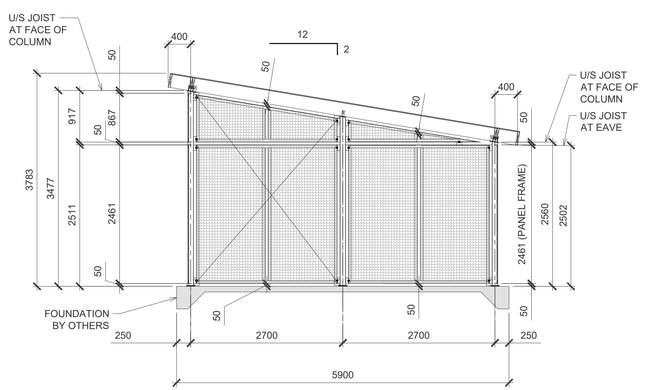

3 S2.01 ELEVATION
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4 S2.01 ELEVATION
SCALE = 1 : 50




5 S2.01 ELEVATION
SCALE = 1 : 50




6 S2.01 ELEVATION
SCALE = 1 : 50

## BEAM SCHEDULE

| TYPE | SIZE | NOTES |
|---|---|---|
| B1 | 38 x 140 | |
| B2 | 38 x 184 | |
| B3 | 38 x 235 | |
| B4 | 38 x 286 | |
| B5 | 44 x 235 LVL | |
| B6 | 44 x 302 LVL | |
| B7 | 44 x 406 LVL | |

NOTE:
- ALL BEAMS TO BE FLUSH FRAMED UNLESS NOTED OTHERWISE. SOLID BEAMS (PSL) SHALL NOT BE SUBSTITUTED WITH LAMINATED BEAMS (LVL) UNLESS APPROVED BY SKYLINE.

## JOIST SCHEDULE

| TYPE | SIZE | NOTES |
|---|---|---|
| J1 | 38 x 140 | SEE PLAN |
| J2 | 38 x 184 | SEE PLAN |
| J3 | 38 x 235 | SEE PLAN |
| J4 | 38 x 286 | SEE PLAN |
| J5 | 241 DP. TJI | |
| J6 | 302 DP. TJI | |

## POST SCHEDULE

| TYPE | SIZE |
|---|---|
| P1 | 38 x 89 |
| P2 | 38 x 140 |
| P3 | 38 x 184 |
| P4 | 89 x 89 |
| P5 | 140 x 140 |

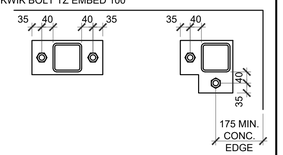
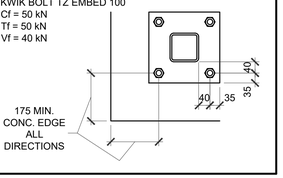
## DECKING SCHEDULE

| TYPE | SIZE |
|---|---|
| D1 | 16mm T&G PLYWOOD SHEATHING |

## STEEL COLUMN SCHEDULE

| TYPE | SIZE |
|---|---|
| SC1 | HSS 102x102x4.8 |

## BASE PLATE SCHEDULE

| TYPE | SIZE |
|---|---|
| BP1 | 16mm PLATE C/W 2 - 16mmØ HILIT KWIK BOLT TZ EMBED 100   |
| BP2 | 16mm PLATE C/W 4 - 16mmØ HILIT KWIK BOLT TZ EMBED 100 Cf = 50 kN Tf = 50 kN Vf = 40 kN   |

NOTE: - ALL STEEL TO BE PRIMED AND PAINTED WITH BLACK POLYURETHANE
- ALL EXPOSED WOOD BEAMS TO BE WRAPPED IN BLACK METAL FLASHING

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PROFESSIONAL ENGINEER — PROVINCE OF BRITISH COLUMBIA — G.W. GRIFFIN — 29547

2026-04-16

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VGH
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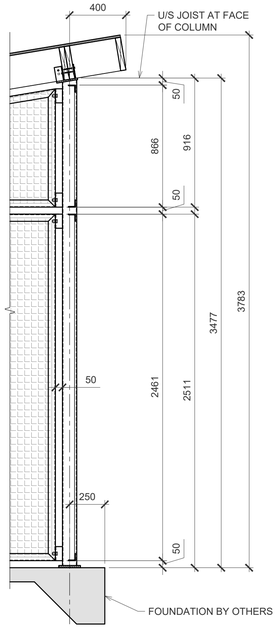
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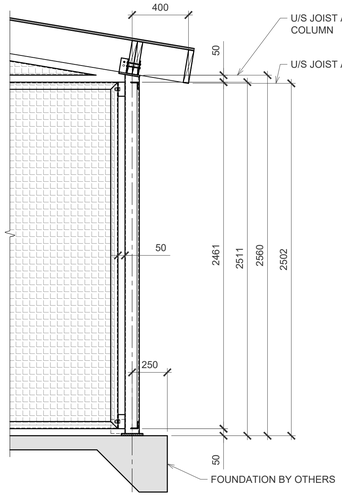
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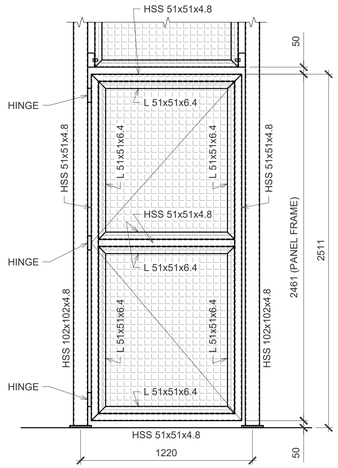
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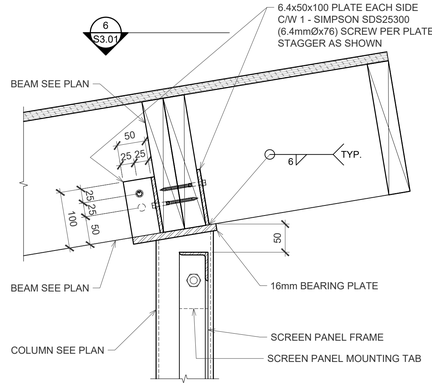

1 S3.01 SECTION
SCALE = 1 : 20



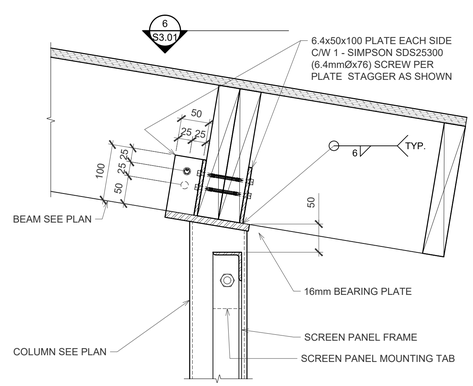

2 S3.01 SECTION
SCALE = 1 : 20



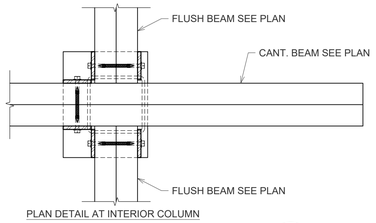

3 S3.01 TYPICAL DOOR DETAIL
SCALE = 1 : 20



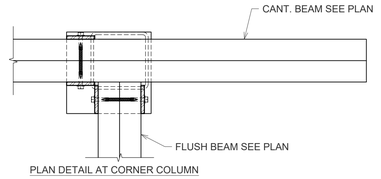

4 S3.01 SECTION
SCALE = 1 : 5




5 S3.01 SECTION
SCALE = 1 : 5

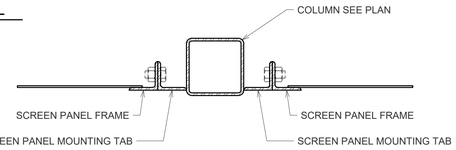


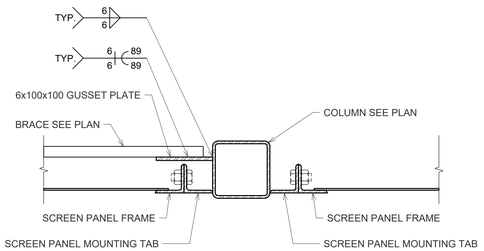

PLAN DETAIL AT INTERIOR COLUMN

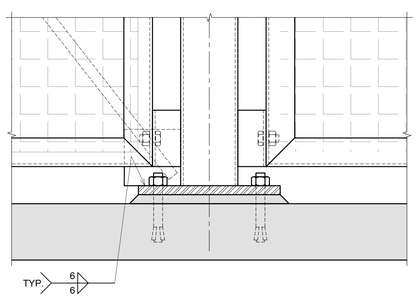



PLAN DETAIL AT CORNER COLUMN

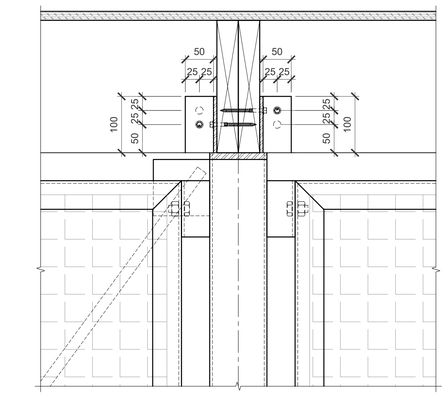
6 S3.01 PLAN DETAIL
SCALE = 1 : 5

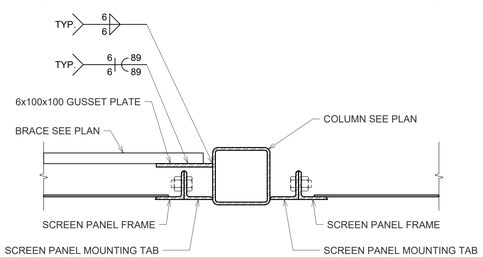


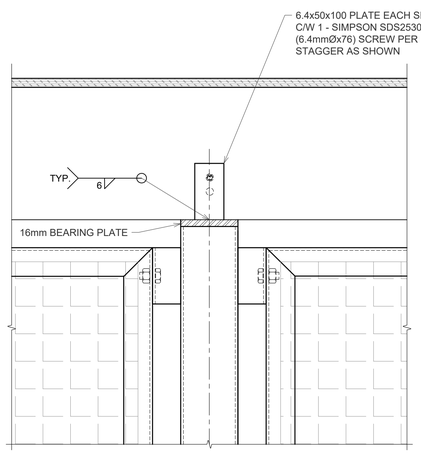




7 S3.01 BRACE CONNECTION AT BOTTOM
SCALE = 1 : 5

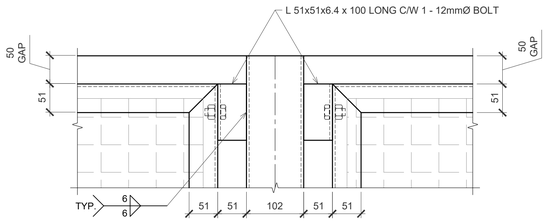


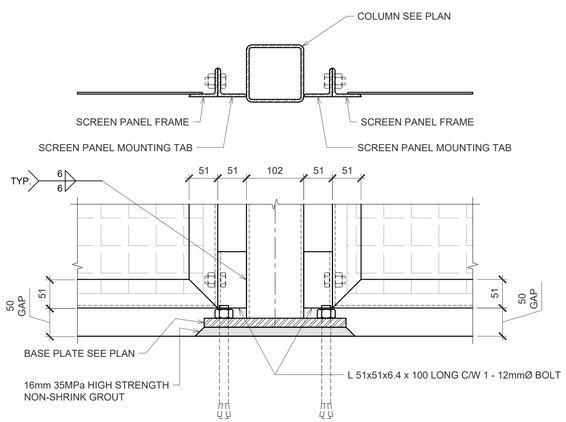

8 S3.01 BRACE CONNECTION AT BOTTOM
SCALE = 1 : 5




9 S3.01 SECTION
SCALE = 1 : 5




10 S3.01 SCREEN PANEL MOUNTING TAB AT TOP
SCALE = 1 : 5




11 S3.01 SCREEN PANEL MOUNTING TAB AT BOT.
SCALE = 1 : 5

SKYLINE ENGINEERING
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PROFESSIONAL ENGINEER
PROVINCE OF BRITISH COLUMBIA
G.W. GRIFFIN

2026-04-16

PROJECT NAME:
VGH
BIKE STORAGE
SHELTER EXPANSION

SHEET TITLE:
SECTIONS

PROJECT NO:
11630.12

SCALE:
AS NOTED

DRAWN: M.E. E.O.R.: W.G.

DRAWING NO:
S3.01

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